



ENERAL NOTES

- A. NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS TAKE PRECEDENCE OVER THESE STANDARD STRUCTURAL NOTES. TYPICAL DETAILS SHALL BE USED WHENEVER APPLICABLE.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEF RTING WORK: AND THE ENGINEER SHALL BE IMMEDIATELY NOTIFIED, IN WRITING, OF ANY
- C. IN NO CASE SHALL DIMENSIONS BE SCALED FROM PLANS, SECTIONS, OR DETAILS ON THE STRUCTURAL DRAWINGS.
- D. THE CONTRACTOR SHALL DETERMINE THE LOCATION OF UTILITY SERVICES IN THE AREA TO BE EXCAVATED BEFORE BEGINNING EXCAVATION.
- E. NO PIPES, DUCTS, SLEEVES, CHASES, ETC., SHALL BE PLACED IN SLABS OR WALLS, NOR SHALL ANY STRUCTURAL MEMBER BE CUT FOR PIPES, DUCTS, ETC.
- THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL TEMPORARY SHORING AND BRACING OF EXISTING STRUCTURAL ELEMENTS DURING CONSTRUCTION, ALL SHORING SHALL BE ADEQUATE TO SUPPORT ALL STRUCTURAL LOADS DURING THE REMOVAL OF THE EXISTING STRUCTURE. TEMPORARY SHORING MUST REMAIN IN PLACE UNTIL ALL NEW STRUCTURAL ELEMENTS ARE SECURED INTO PLACE PER CONSTRUCTION DOCUMENT.
- REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR REQUIREMENTS, DIMENSIONS AND EXACT LOCATIONS OF FLOOR DRAINS, TRENCHES, DRAIN TILE, PUMPS AND EQUIPMENT INCLUDING ANORONING SYSTEMS AND HOUSEKEEPING FADS. GENERAL CONTRACTOR TO COORDINATE ALL OF THESE ITEMS WITH ALL DISCIPLINES INVOLVED
- H. ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF THE FOLLOWING CODES AND MANUALS (LATEST ADOPTED EDITION):

 1. STATE BUILDING CODE, WHEN APPLICABLE:

 2. INTERNATIONAL BUILDING CODE (BIGC).

 3. AMERICAN CONCRETE INSTITUTE (ACI).

 4. CONCRETE REINFORCING STEEL INSTITUTE (CRSI) MANUAL OF STANDARD PRACTICE (FOR PLACING AND DETAILING OF ALL REINFORCING).

 5. AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC).

 6. AMERICAN WELDING SOCIETY (AWS) STANDARDS FOR WELDING AS MODIFIED BY AISC SPECIFICATION.

- SPECIFICATION SPECIFICATION.

 7. MASONRY STANDARDS JOINT COMMITTEE (MSJC)

 8. AMERICAN FOREST & PAPER ASSOCIATION NATIONAL DESIGN SPECIFICATION (AF & PA NDS)

ESIGN LOADS CRITERIA

- 2015 INTERNATIONAL BUILDING CODE
 2010 AMERICAN SOCIETY OF CIVIL ENGINEERS STANDARD 7 (ASCE 7-10)

- ULTIMATE WIND SPEED, V = 115 MPH (3 SECOND GUST) NOMINAL WIND SPEED, V = 90 MPH (3 SECOND GUST)

- 3. WIND LOAD EXPOSURE: C
 4. INTERNAL PRESSURE COEFFICIENT: +/-0.00 (OPEN BUILDING)
 5. WIND TOPOGRAPHIC FACTOR: K_{2T} = 1.0
 6. C & C WIND WALL PRESSURE: REFER TO COMPONENT AND CLADDING WIND PRESSURE TABLE
- D. SNOW LOAD CRITERIA:

 1. GROUND SNOW LOAD, P_G = 30 PSF

 2. FLAT-ROOF SNOW LOAD (BALANCED), P_F = 25.2 PSF

- FLA1-HOUP SNUW LOAD (BALANCED), $P_T=25.2$ PSF SNOW LOAD IMPORTANCE FACTOR, $I_0=1.0$ SLOPE FACTOR, $C_0=1.0$ THERMAL FACTOR, $C_T=1.2$ SNOW EXPOSURE FACTOR, $C_E=1.0$ UNBALANCED SNOW LOAD: ON PLAN IF APPLICABLE
- EARTHQUAKE LOAD CRITERIA

 1. SEISMIC IMPORTANCE FACTOR: IE = 1.0 MAPPED SPECTRAL RESPONSE ACCELERATIONS:

- . MAPPED SPECTRAL RESPONSE ÁCCELERATIONS:

 . \$a = 10 %g

 . \$s = 5.1% g

 . \$0 = 5.107

 . \$0 = 0.082

 . \$EISMIC DESIGN CATEGORY = B

 . \$EISMIC PORCE RESISTING SYSTEM: STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED

 FOR SEISMIC RESISTANCE.

 SEISMIC SEPROMSE OFERFICIENT Cx = 0.036
- SEISMIC RESPONSE COEFFICIENT, Cs = 0.036 RESPONSE MODIFICATION FACTOR, R = 3

- OVER-STRENGTH FACTOR: Q = 3
 OVER-STRENGTH FACTOR: Q = 3
 O. DESIGN BASE SHEAR, V = 0.036 W
 II. ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL-FORCE ANALYSIS

- F. LIVE LOADS

 1. ROOF: 20 PSF

 2. FLOOR: 50 PSF

 3. RAILING: 200 LBS AT ANY POINT OR 50 PLF, WHICHEVER PRODUCES MAXIMUM LOAD EFFECT

- G. DEAD LOADS
 1. ROOF: 10 PSF
 2. FRAMING: ACTUAL
 3. MISCELLANEOUS CEILING: 6 PSF
 4. MECHANICAL: SEE PLAN

- SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR APPROVAL, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER RESPONSIBLE FOR ITS PREPARATION, WHO IS REGISTERED IN THE STATE WHICH THE PROJECT IS LOCATED.
- B. PRIOR TO SUBMITTAL, THE CONTRACTOR SHALL REVIEW THE SHOP DRAWINGS AND MAKE ANY CORRECTIONS REQUIRED. THE CONTRACTOR SHALL STAMP AND SIGN THE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ENGINEER.
- C. THE ENGINEER'S REVIEW OF SHOP DRAWINGS IS FOR GENERAL CONFORMANCE OF THE DESIGN CONCEPT. CONTRACTOR SHALL SUBMIT A SCHEDULE OF SHOP DRAWING SUBMITTALS THAT IS ACCEPTABLE TO BOTH CONTRACTOR AND ENGINEER. AFTER THE CONT
- D. FOR LARGE SUBMITTALS, REASONABLE REVIEW TIME SHALL BE ALLOWED AND MAY EXCEED TWO WEEKS. THE CONTRACTOR SHALL SUBMIT NECESSARY REQUEST FOR INFORMATION (RFIs) DURING THE DETAILING PROCESS TO AVOID SUBMITTALS THAT ARE INCOMPLETE OR NEED SIGNIFICANT VERHICATIONS. THE CONCURRENT SUBMITTAL OF MULTIPLE SHOP DRAWINGS (DUMPING) WILL FURTHER EXTEND THE REVIEW PROCESS AND TIME FRAME INCESSARY TO PROPERLY REVIEW EACH
- E. UNLESS INDICATED OTHERWISE, THE GENERAL CONTRACTOR SHALL SUBMIT SHOP DRAWINGS OF THE FOLLOWING ITEMS FOR STRUCTURAL REVIEW. REFER TO SPECIFIC SECTION OF STRUCTURAL NOTES FOR ANY ADDITIONAL CRITERIA:
- CONCRETE MIX DESIGN:
- PRE-ENGINEERED METAL BUILDING DESIGN
 ADDITIONAL STRUCTURAL SHOP DRAWINGS REQUESTED IN THE SPECIFICATIONS
- F. A COPY OF ALL SHOP DRAWINGS SHALL BE MAINTAINED ON SITE AT ALL TIMES.
- G. SHOP DRAWINGS SHALL INCLUDE COMPLETE DETAIL SCHEDULES, PROCEDURES, AND DIAGRAMS FOR FABRICATION AND ASSEMBLY OF STRUCTURAL MEMBERS AND SUBMIT PRIOR TO FABRICATION.
- H. ERECTION PLANS ARE THE RESPONSIBILITY OF THE FABRICATOR

FOOTINGS AND FOUNDATIONS

- A. SOIL BEARING DESIGN VALUE:
 1. 1550 PSF (PRESUMED).
 2. BEARING VALUE TO BE VERIFIED IN FIELD BY GEOTECHNICAL ENGINEER.
- B. PROTECT FOUNDATION EXCAVATIONS FROM FROST; DO NOT PLACE CONCRETE ON FROZEN GROUND.
- C. FOUNDATION EXCAVATIONS SHALL BE KEPT FREE OF LOOSE MATERIAL AND STANDING WATER AND SHALL BE CHECKED AND APPROVED BY THE ENGINEER BEFORE THE PLACEMENT OF ANY CONCRETE.
- D. DESIGN FROST PENETRATION DEPTH: 42 INCHES (HEATED) OR 60 INCHES (UNHEATED)
- E. MINIMUM OF 6" COMPACTED GRANULAR SUBGRADE BELOW SLABS

MATERIAL COMPACTION CRITERIA		
LOCATION	MINIMUM RELATIVE COMPACTION PERCENTAGE (ASTM D698 STANDARD PROCTOR DENSITY (SPD))	
1'-0" BELOW FOUNDATION AND SLAB SUBGRADE ELEVATIONS	98%	
ABOVE BOTTOM OF FOUNDATIONS AND BELOW SLAB SUBGRADE ELEVATIONS	95%	
BELOW EXTERIOR SLAB, WITHIN 1'-0" OF SUBGRADE ELEVATIONS	98%	
BELOW EXTERIOR SLAB, MORE THAN 1'-0" BELOW SUBGRADE ELEVATIONS	95%	

A. CONCRETE SHALL BE STANDARD WEIGHT MIX UNLESS NOTED OTHERWISE AND MEET THE FOLLOWING

LOCATIONS	fc @ 28 DAYS	AIR ENTRAINMENT	MAX. WATER/CEMENT RATIO
FOOTINGS / FOUNDATIONS	4000 PSI	6% ± 1.5%	0.50
FLOORS ON GRADE	4000 PSI		0.45
COLUMNS	4000 PSI		0.45
EXTERIOR SLABS ON GRADE	4500 PSI	6% ± 1.5%	0.45
EXPOSED EXTERIOR WALLS	4500 PSI	6% ± 1.5%	0.45

- B. CEMENT SHALL CONFORM TO ASTM C150, TYPE I / II OR ASTM C595 TYPE IL
- C. READY-MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCO
- D. CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301 (LATEST EDITION)
 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS", EXCEPT AS MODIFIED BY THESE
- E. ADMIXTURES MAY BE USED WITH PRIOR APPROVAL OF THE ENGINEER. ADMIXTURES SHALL COMPLY WITH ASTIM C494 AND BE OF A TYPE THAT INCREASES THE WORKABILITY OF THE CONCRETE, BUT SHALL NOT BE CONSIDERED TO REDUCE THE SPECIFIED MINIMUM CEMENT CONTENT (CALCIUM CHLORIDE SHALL NOT BE USED).
- F. CONTRACTOR SHALL SUBMIT MIX DESIGNS FOR APPROVAL 10 DAYS PRIOR TO FABRICATION AND INSTALLATION. ALL CONCRETE MIXES SHALL BE DESIGNED AND CERTIFIED BY A MATERIALS TESTING
- G. PROJECTING CORNERS OF SLABS, BEAMS, WALLS, COLUMNS, ETC. SHALL BE FORMED WITH A 3/4" CHAMFER UNLESS DETAILED OR NOTED OTHERWISE.
- H. PLACE VAPOR RETARDER OR VAPOR BARRIER DIRECTLY BELOW FLOOR SLAI
- I. CONCRETE FLOOR SHALL BE CURED IN ACCORDANCE WITH ASTM C309. CONCRETE FLOOR SHALL BE PROTECTED FROM MOISTURE LOSS FOR A MINIMUM OF 14 DAYS, USING AN APPROVED SHEET
- J. FLOOR FLATNESS AND LEVELNESS TOLERANCES:
 1. UNLESS NOTED OTHERWISE, FLOORS SHALL CONFORM TO THE FOLLOWING SURFACE PROFILE
- FLOOR TOLERANCE (F_F AND F_L) MEASUREMENTS SHALL BE TESTED IN ACCORDANCE WITH ASTM E 1155. ACTUAL OVERALL F-NUMBERS SHALL BE CALCULATED USING THE INFERIOR / SUPERIOR AREA
- METHOD.

 3. CORRECT DEFECTIVE SLABS BY GRINDING OR REMOVING AND REPLACING DEFECTIVE WORK. RE-MEASURE CORRECTED AREAS BY THE SAME PROCESS.

- A. ALL ANCHOR RODS SHALL BE SUPPLIED AND INSTALLED BY THE CONCRETE CONTRACTOR, UNLESS NOTED OTHERWISE.
- B. ALL ANCHOR RODS SHALL BE ASTM F1554 GRADE 36 HEX-HEAD, UNLESS NOTED OTHERWISE. NI SHALL BE ASTM A563 GRADE A HEAVY HEX. OVER-SIZED PLATE WASHERS SHALL BE ASTM A36.
- C. ALL ANCHOR RODS SHALL BE SET WITH TEMPLATES.
- D. POST-INSTALLED ANCHORS SHALL BE ADHESIVE ANCHORING SYSTEM PROVIDED AND INSTALLED BY POST-INSTALLED ANCHORS SHALL BE ADHESIVE ANCHORING STSTEM PROVIDED AND IN STALLED BY FRAMING CONTRACTOR. ADHESIVE ANCHORS SHALL BE "HILTI HIT-HY 200 ADHESIVE ANCHOR SYSTEM" OR APPROVED ALTERNATE. ANCHORS SHALL BE "HILTI HAS-E" THREADED ROD CONFORMING TO ISO 889-1 CLASS S.6 OR SHALL BE MADE FROM ALL-THREADED ROD CONFORMING TO ASTM A572 GRADE 60, OR APPROVED ALTERNATE, UNLESS NOTED OTHERWISE.

REINFORCING STEEL

- B. MINIMUM DEVELOPMENT LENGTH OF REINFORCING BARS SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE.

MINIMUM LENGTH FOR STANDARD UN-COATED BARS IN NORMAL WEIGHT CONCRETE						
DEVELOPMENT LENGTH (Ld) FOR STRAIGHT BARS (MIN. OF 12 INCHES)				FOR 90 DEGREE		
CONCRETE STRENGTH	TENSION CLASS A		TENSION CLASS B		COMPRESSION	HOOKED BARS,
fc IN PSI	#6 AND SMALLER	#7 TO #11	#6 & SMALLER	#7 TO #11	#18, #14, & #11 AND SMALLER	DEVELOPMENT LENGTH
3000	44 Db	55 Db	57 Db	71 Db	30 Db	22 Db
3500	41 Db	51 Db	53 Db	66 Db	30 Db	20 Db
4000	38 Db	47 Db	49 Db	62 Db	30 Db	19 Db
4500	36 Db	45 Db	47 Db	58 Db	30 Db	18 Db
5000	34 Db	42 Db	44 Db	55 Db	30 Db	17 Db
NOTE: Db = DIAMETER OF REINFORCEMENT. Ld = DEVELOPMENT LENGTH						

- LIGHTWEIGHT CONCRETE = 1.3.

- 2. EPOXY COATED #11.2. LEPOXY COATED WITH COVER LESS THAN 3DB OR CLEAR SPACING LESS THAN 6 DB = 1.5. HORIZONTAL TOP BARS WITH 12 OF CONCRETE CAST BELOW = 1.3. LEPOXY COATED HORIZONTAL TOP BARS WITH 12 OF CONCRETE CAST BELOW = NOT GREATER
- THAN 1.7.

 E. ADJUSTMENT FACTORS FOR STRAIGHT HOOKS IN TENSION

 1. LIGHTWEIGHT CONCRETE = 1.3.

 2. EPOXY COATED = 1.2.
- REINFORCING STEEL SHALL BE PROVIDED WITH THE FOLLOWING AMOUNTS OF COVER FOR CAST-IN-PLACE CONCRETE UNLESS NOTED OTHERWISE:

COLUMNS AND PIERS (COVER TO STIRRUPS AND TIES)

MINIMUM CLEAR CONCRETE COVER FOR REINFORCING STEEL		
CONCRETE ON SOIL (DIRECT CONTACT)	3"	
SLAB ON GRADE	CENTERED	
WALLS, STRUCTURAL SLABS EXPOSED TO SOIL OR WEATHER #6 TO #18 REBAR	2"	
#5 AND SMALLER REBAR	1 1/2"	
WALLS, STRUCTURAL SLABS NOT EXPOSED TO EARTH OR WEATHER		

- G. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS, AND INSERTS SHALL BE SECURED IN POSITION WITH WIRE POSITIONERS, OR EQUAL, BEFORE PLACING CONCRETE OR GROUT.
- H. DOWELS BETWEEN FOOTINGS AND WALLS SHALL BE THE SAME GRADE, SIZE, AND SPACING AS VERTICAL WALL REINFORCING.
- CONTRACTOR SHALL SUBMIT REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL A MINIMUM OF 10 DAYS PRIOR TO FABRICATION AND INSTALLATION.
- J. BARS TO BE WELDED SHALL BE ASTM A706, GRADE 60. WELDING OF REINFORCING BARS SHALL CONFORM TO AWS D1.4.

PRE-ENGINEERED METAL BUILDING

- A. THE ENTIRE PRE-ENGINEERED METAL BUILDING SYSTEM SHALL BE DESIGNED BY THE METAL BUILDING MANUFACTURER IN CONFORMANCE TO THE PROVISIONS OF THE INTERNATIONAL BUILDING CODE AND THE METAL BUILDING SYSTEMS MANUFACTURERS ASSOCIATION. WHERE THESE CRITERIA CONFLICT, THE MORE STRINGENT CRITERIA SHALL APPLY.
- B. IT IS THE PRE-ENGINEERED MANUFACTURER'S RESPONSIBILITY TO DESIGN THE COMPLETE BUILDING SYSTEM (STEEL FRAMING, ANCHOR BOLTS, COMPONENTS, ATTACHMENTS, ETC.) THE MANUFACTURER SHALL SUBMIT A CERTIFICATION LETTER BEARING THE SEAL OF A PROFESSIONAL ENGINEER STATING THAT THE BUILDING SYSTEM DESIGN MEETS THE INDICATED CODE, PERFORMANCE AND LOADING REQUIREMENTS.
- C. THE PRE-ENGINEERED METAL BUILDING MANUFACTURER SHALL BE CERTIFIED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, CATEGORY MB. THE MANUFACTURER SHALL MEET THE EXCEPTION NOTED PER THE IBC WHICH SPECIFIES QUALITY CONTROL REQUIREMENTS OF THE MANUFACTURER PERTAINING TO SPECIAL INSPECTIONS.
- D. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS OF THE ENTIRE METAL BUILDING SYSTEM FOR REVIEW. THE CONTRACTOR SHALL ALSO SUBMIT A COMPLETE STRUCTURAL DESIGN ANALYSIS OF THE BUILDING SYSTEM FOR RECORD PURPOSES ONLY). THE SHOP DRAWING SUBMITTAL SHALL INCLUDE ALL ANCHOR BOLT REQUIREMENTS AND FOUNDATION REACTIONS. ALL SHOP DRAWINGS AND CALCULATION SUBMITTALS SHALL BEAR THE SEAL OF A PROFESSIONAL ENGINEER.
- E. DESIGN LOADS TO BE USED IN CONNECTION WITH THE METAL BUILDING DESIGN ARE PER THE DESIGN LOADS CRITERIA. IN ADDITION TO THE ACTUAL DEAD LOAD, AN ADDITIONAL COLLATERAL ROOF FRAMING DEAD LOAD OF 5 PSF SHALL BE INCLUDED. COORDINATE ANY EQUIPMENT LOADS WITH THE MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS. F. CALCULATIONS FOR FRAME DEFLECTIONS SHALL BE DONE USING ONLY THE BARE FRAME METHOD. REDUCTIONS BASED ON ENGINEERING JUDGMENT USING THE ASSUMED COMPOSITE STIFFNESS OF THE BUILDING ENVELOPE SHALL NOT BE PERMITTED. DRIFT SHALL FOLLOW AISC'S "SERVICEABILITY DESIGN CONSIDERATIONS FOR LOW-RISE BUILDINGS." CALCULATIONS SHALL BE SUBMITTED VERIFYING THAT THE ACTUAL DRIFT UNDER CODE REQUIRED LOADINGS DOES NOT EXCEED THE MINISTER.
- G. THE PRE-ENGINEERED MANUFACTURER SHALL PROVIDE ALL GIRTS, PURLINS, FRAMES AND OTHER COMPONENTS REQUIRED FOR A COMPLETE SYSTEM. ALL WALL SYSTEMS, SUCH AS METAL STUDS, STOREFRONTS, ETC. SHALL BE PROPERLY SUPPORTED BY THE METAL BUILDING SYSTEM. ALLOWABLE DEFLECTIONS OF COMPONENTS SHALL BE IN ACCORDANCE WITH THE IBC.
- H. THE FOUNDATION DESIGN IS BASED UPON THE PRE-ENGINEERED METAL BUILDING SYSTEM REACTIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ANY REVISIONS REQUIRED AS A RESULT OF CHANGE IN THE BUILDING MANUFACTURER, INCLUDING REDESIGN OF THE FOUNDATIONS. I. THE SIZE, NUMBER AND PLACEMENT PATTERN OF ALL ANCHOR BOLTS SHALL BE DETERMINED BY THE PRE-ENGINEERED BUILDING MANUFACTURER. ANCHOR BOLT EMBEDMENTS ARE INDICATED ON THE
- STRUCTURAL DRAWINGS AND DETAILS. J. THE PRE-ENGINEERED METAL BUILDING SHALL BE DESIGNED BY THE MANUFACTURER TO RESIST LATERAL LOADS AS FOLLOWS:

 a. INTERIOR FRAME LINES: RIGID FRAMES (PINNED-BASED COLUMNS)

 b. PERIMETER WALL LINES: BRACED BAYS OR PORTAL FRAMES
- K. THE METAL BUILDING ERECTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING
- L. UNLESS NOTED OTHERWISE OR SPECIFIED, ALL STEEL MEMBERS SHALL BE CLEANED AND PAINTED IN ACCORDANCE WITH THE MANUFACTURER'S STANDARD PROCEDURES.



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CONSERV FS KANSASVILLE

KANSASVILLE WISCONS REVISION SCHEDULE DESCRIPTION

23-28114 ROJECT NO DESIGNED BY OA REVIEWED BY DCM RIGINAL ISSUE DATE 09/22/23

TITLE

CLIENT PROJECT NO

STRUCTURAL NOTES

S1-00

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SPECIAL INSPECTIONS

- 1. SPECIAL INSPECTION PROGRAM SHALL CONFORM TO CHAPTER 17 OF THE IBC.
- THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR TO PERFORM THE REQUIRED TESTS AND SPECIAL INSPECTIONS WITH QUALIFICATIONS DESCRIBED PER IBC CHAPTER 17 AND THE PROJECT SPECIFICATIONS.
- THE CONTRACTOR IS RESPONSIBLE FOR SCHEDULING INSPECTIONS AND TESTS. SUFFICIENT NOTICE AND LEAD TIME MUST BE ALLOWED FOR THE INSPECTION AND TESTING TO BE PERFORMED WITHOUT IMPEDING CONSTRUCTION OPERATIONS.
- SPECIAL INSPECTION REPORTS SHALL BE FURNISHED TO BUILDING OFFICIAL, OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND CONTRACTOR.
- 5. WHEN DEFICIENCIES ARE IDENTIFIED, THE CONTRACTOR MUST TAKE CORRECTIVE ACTIONS TO COMPLY WITH THE CONTRACT DOCUMENTS OR REMEDY THE DEFICIENCIES AS DIRECTED BY THE REGISTERED DESIGN PROFESSIONAL.
- 6. THE SPECIAL INSPECTION AND QUALITY ASSURANCE PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF HIS OR HER RESPONSIBILITY TO PERFORM QUALITY CONTROL.
- 7. THE CONTRACTOR IS RESPONSIBLE FOR TESTING SERVICES THAT ARE REQUIRED FOR MATERIAL SUBMITTALS AND THAT ARE NOT PART OF THE SPECIAL INSPECTION PROGRAM (E.G. AGGREGATE TESTS, CONCRETE MIX DESIGNS, TESTING OF CONTROLLED FILL MATERIALS, ETC.).
- SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT STATING THAT THE STRUCTURAL WORK WAS, TO THE BEST OF THE SPECIAL INSPECTOR'S KNOWLEDGE, PERFORMED IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.

FABRICATED ITEMS (IBC 1704.2)

SPECIAL INSPECTION TYPE: FABRICATOR APPROVAL

SPECIAL INSPECTION IS NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION PROVIDED THE FABRICATOR COMPLIES WITH BC.

CAST-IN-PLACE CONCRETE (IBC 1705.3)

CAST-IN-PLACE CONCRETE (IBC 1703.	
SPECIAL INSPECTION TYPE	FREQUENCY
INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	PERIODIC
REINFORCING BAR WELDING:	
VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706.	PERIODIC
INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16".	PERIODIC
INSPECT ALL OTHER WELDS.	CONTINUOUS
INSPECT ANCHORS CAST IN CONCRETE.	PERIODIC
INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.	
ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	CONTINUOUS
MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED ABOVE.	PERIODIC
VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC
INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS
VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC
INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	PERIODIC
FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	CONTINUOUS AT TIME OF TESTING
SAMPLE FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN NOT LESS THAN ONCE A DAY, NOR LESS THAN ONCE FOR EACH 150 CUBIC YARDS OF CONCRETE, NOR LESS THAN ONCE FOR EACH 5,000 SQUARE FEET OF SURFACE AREA FOR SLABS OR WALLS. A MINIMUM OF FIVE STRENGTH TESTS SHOULD BE MADE FOR A GIVEN PROJECT.	

SOILS (IBC 1705.6)

SPECIAL INSPECTION TYPE	FREQUENCY
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	PERIODIC
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	PERIODIC
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	PERIODIC
VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	CONTINUOUS
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIEY THAT SITE HAS BEEN PREPARED PROPERLY	PERIODIC

SPECIAL INSPECTIONS FOR WIND RESISTANCE (IBC 1705.11)

SPECIAL INSPECTION TYPE	FREQUENC
WIND-RESISTING COMPONENTS:	
INSPECT FASTENING OF THE ROOF COVERING, ROOF DECK, AND ROOF FRAMING CONNECTIONS.	PERIODIC
INSPECT FASTENING OF THE EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING.	PERIODIC



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PROJECT

CONSERV FS KANSASVILLE

KANSASVILLE WISCONSIN

REVISION SCHEDULE

DATE DESCRIPTION BY

PROJECT NO.	23-28114
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DRAWN BY	KNM
DESIGNED BY	OA
REVIEWED BY	DCM
ORIGINAL ISSUE DATE	09/22/23
CLIENT PROJECT NO.	

TITLE

SPECIAL INSPECTIONS (PER IBC 2015)

SHEET

S1-01